

Answer all Questions

Q1 (50 %): Check the two way shear action (punching shear) only around an edge Column (300×300) mm in a flat plate floor of a span (5.5 × 5.5) m. Find the area of vertical shear reinforcement if required. Assume $d = 190$ mm. Total $q_u = 25$ kPa (including slab weight), $f_c' = 28$ MPa, $f_y = 414$ MPa.

Q2 (50 %): for the longitudinal interior frame (A) of the flat plate floor shown in figure below by using direct design method find:

1. Longitudinal distribution of the static moment at factored loads.
2. Lateral distribution of the moment at exterior support.

Slab thickness = 170 mm, $d = 144$ mm

$q_u = 20.0$ kN/m²

All edge beams = 350 × 700 mm

All columns = 500 × 500 mm

$f_c' = 25$ Mpa, $f_y = 420$ Mpa

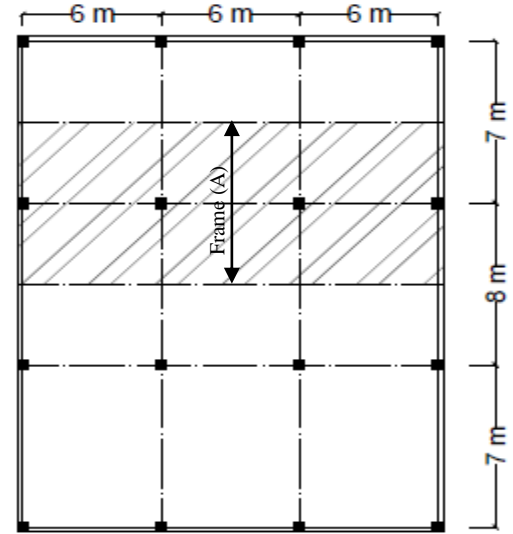


Table 8.10.4.2—Distribution coefficients for end spans

| | Exterior edge unrestrained | Slab with beams between all supports | Slab without beams between interior supports | | Exterior edge fully restrained |
|-------------------|----------------------------|--------------------------------------|--|----------------|--------------------------------|
| | | | Without edge beam | With edge beam | |
| Interior negative | 0.75 | 0.70 | 0.70 | 0.70 | 0.65 |
| Positive | 0.63 | 0.57 | 0.52 | 0.50 | 0.35 |
| Exterior negative | 0 | 0.16 | 0.26 | 0.30 | 0.65 |

$$C = \Sigma \left(1 - 0.63 \frac{x}{y} \right) \frac{x^3 y}{3} \quad \beta_t = \frac{E_{cb} C}{2 E_{cs} I_s}$$

Table 8.10.5.1—Portion of interior negative M_u in column strip

| $\alpha_1 \ell_2 / \ell_1$ | ℓ_2 / ℓ_1 | | |
|----------------------------|-------------------|------|------|
| | 0.5 | 1.0 | 2.0 |
| 0 | 0.75 | 0.75 | 0.75 |
| ≥ 1.0 | 0.90 | 0.75 | 0.45 |

Note: Linear interpolations shall be made between values shown.

Table 8.10.5.2—Portion of exterior negative M_u in column strip

| $\alpha_1 \ell_2 / \ell_1$ | β_t | ℓ_2 / ℓ_1 | | |
|----------------------------|------------|-------------------|------|------|
| | | 0.5 | 1.0 | 2.0 |
| 0 | 0 | 1.0 | 1.0 | 1.0 |
| | ≥ 2.5 | 0.75 | 0.75 | 0.75 |
| ≥ 1.0 | 0 | 1.0 | 1.0 | 1.0 |
| | ≥ 2.5 | 0.90 | 0.75 | 0.45 |

Note: Linear interpolations shall be made between values shown. β_t is calculated using Eq. (8.10.5.2a), where C is calculated using Eq. (8.10.5.2b).

Table 8.10.5.5—Portion of positive M_u in column strip

| $\alpha_1 \ell_2 / \ell_1$ | ℓ_2 / ℓ_1 | | |
|----------------------------|-------------------|------|------|
| | 0.5 | 1.0 | 2.0 |
| 0 | 0.60 | 0.60 | 0.60 |
| ≥ 1.0 | 0.90 | 0.75 | 0.45 |

Note: Linear interpolations shall be made between values shown.

| | |
|--|---|
| v_c with non shear reinforcement is least of | $0.33 \lambda \sqrt{f_c'}$ |
| | $0.17 \left(1 + \frac{2}{\beta} \right) \lambda \sqrt{f_c'}$ |
| | $0.083 \left(2 + \frac{\alpha_s d}{b_o} \right) \lambda \sqrt{f_c'}$ |
| v_c with shear reinforcement | $v_c = 0.17 \lambda \sqrt{f_c'}$ |
| Maximum v_u with shear reinforcement | $v_u = \phi 0.5 \lambda \sqrt{f_c'}$ |
| Shear resist by reinforcement | $v_s = \frac{v_u}{\phi} - v_c = \frac{A_v f_y}{b_o s}$ |

Note: β is the ratio of long side to short side of the column
 $\lambda = 1$ for normal concrete, $\alpha_s = 40$ for interior column,
 $\alpha_s = 30$ for edge column & $\alpha_s = 20$ for corner column